

## Analysis of deformation and pile group dimensioning

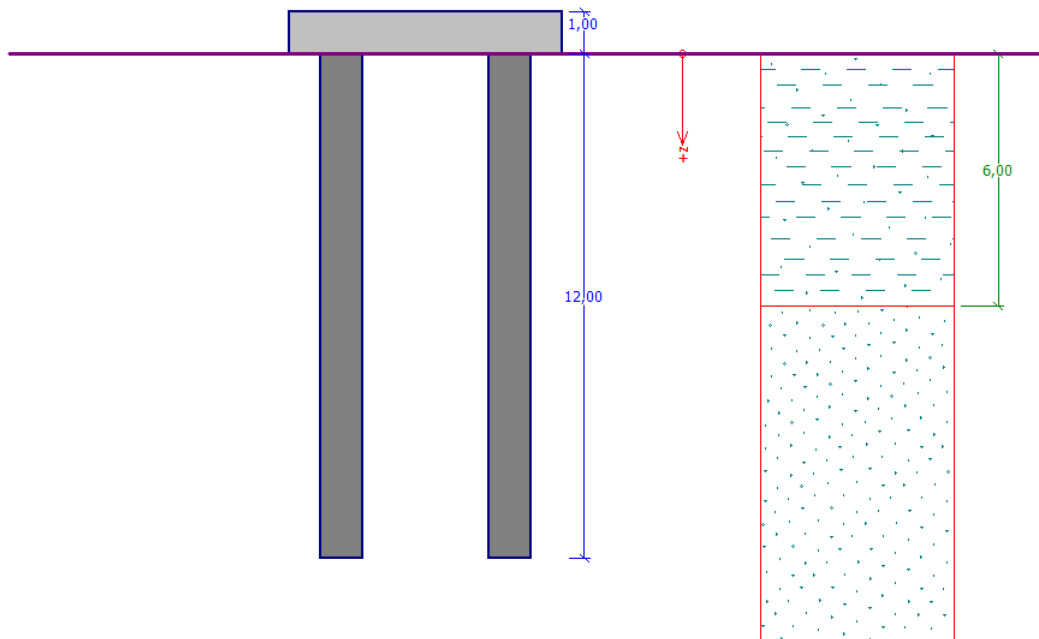
Program: Pile Group

Soupor: Demo\_manual\_18.gsp

The objective of this chapter is to explain the usage of the GEO 5 – PILE GROUP program to analyse the angular rotation and displacement of a stiff pile cap and to determine the internal forces acting along the lengths of individual piles and to the pile cross-sections dimensioning.

### Problem specification

General specification of the problem is described in the previous chapter (*12. Pile foundations – Introduction*). All analyses of the vertical load-bearing capacity of a pile group shall be carried out relative to the previous problem *17. Analysis of vertical bearing capacity and settlement of pile group*. The resultant of the total load comprising  $N, M_y, H_x$  acts at the upper base of the pile cap, right at its centre. The dimensioning of piles in the group shall be carried out in accordance with standard EN 1992-1-1 (EC 2), using standard values of partial coefficients.



*Problem specification chart – pile group*

## Solution

To solve this problem, we will use the GEO 5 – PILE GROUP program. To simplify the problem and quicken the settings of general parameters of the problem we will use all input data from the problem 17. *Analysis of vertical load-bearing capacity of pile group* (e.g. by means of data importing).

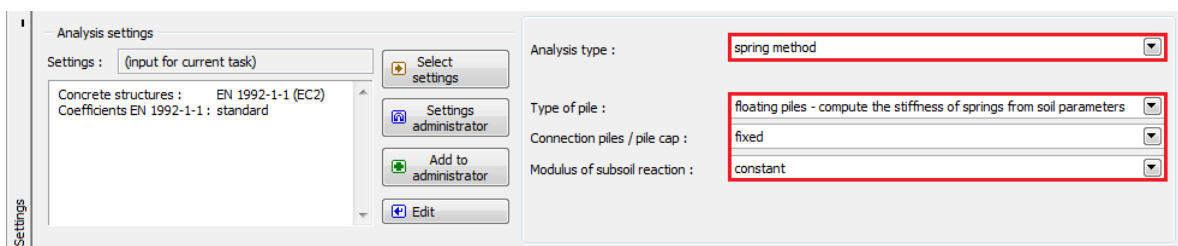
We will analyse the pile group using the so-called *Spring Method*, which models individual piles as beams on an elastic bed. Each pile is internally divided into ten sections, for which the values of horizontal and vertical springs are computed. The base slab (pile cap) is considered to be infinitely stiff. The solution itself is carried out using the deformation variant of the Finite Element Method.

## Specification procedure

We will change the analysis type in the “Settings” frame to the “Spring method” option. We will consider the connection of piles to the base slab to be **stiff, i.e. fixed**. It is assumed for this boundary condition that the bending moment will be transferred in the pile heads.

For the pile bearing at the base we will select the “floating piles – compute the stiffness of springs from soil parameters” option.

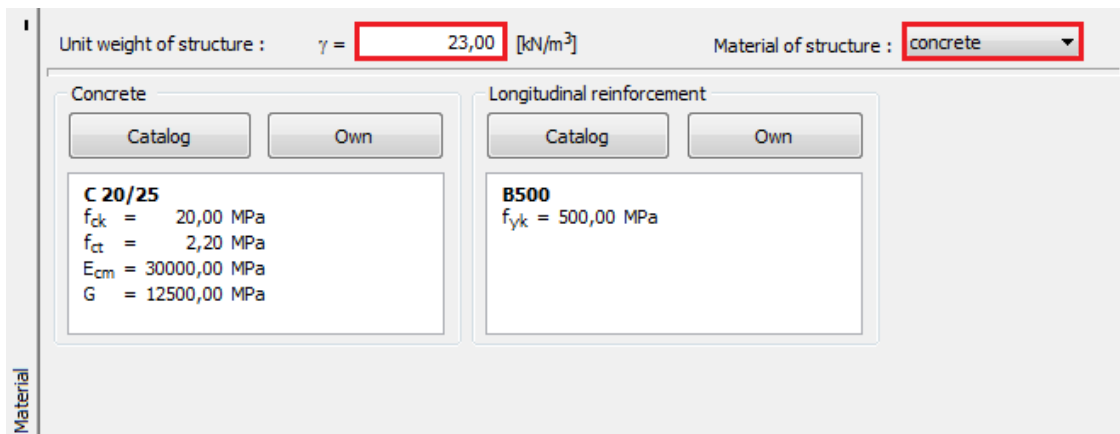
*Note: The program makes several options for boundary conditions for the pile bearing in vertical direction possible. For end-bearing piles, or piles keyed into bedrock, the vertical stiffness of springs is not specified – the pile base is modelled as a joint or a sliding joint. For floating piles, it is necessary to define the sizes of vertical springs, both on the skin and then on the pile base. The program makes specifying the size of springs possible, but it is mostly reasonable to select the “compute the size of springs” option. In this case the program computes the springs from deformational properties of soils for the typical load set (for more details visit Help – F1).*



“Analysis settings” frame – spring method

The horizontal modulus of subsoil reaction characterises the pile behaviour in the lateral direction. In this analysis we will consider the modulus  $k_h$  (inclusive of parameters affecting its magnitude) to be identical with that used in the single pile solution (see Chapter 13. *Analysis of horizontal bearing capacity of a single pile*). In the opening part of this chapter we will carry out the analysis using the constant modulus of subsoil reaction and, in the second part, we will compare the differences between the results when other methods are used (linear – according to Bowles, according to CSN 73 1004 and according to Vesic).

In the “Material” frame we will specify the characteristics of individual piles in the group, i.e. the unit weight of the structure, the concrete type used and longitudinal concrete reinforcement for the dimensioning of the pile shaft.



Unit weight of structure :  $\gamma = 23,00$  [kN/m<sup>3</sup>]      Material of structure : concrete

**Concrete**

Catalog      Own

**C 20/25**  
 $f_{ck} = 20,00$  MPa  
 $f_{ct} = 2,20$  MPa  
 $E_{cm} = 30000,00$  MPa  
 $G = 12500,00$  MPa

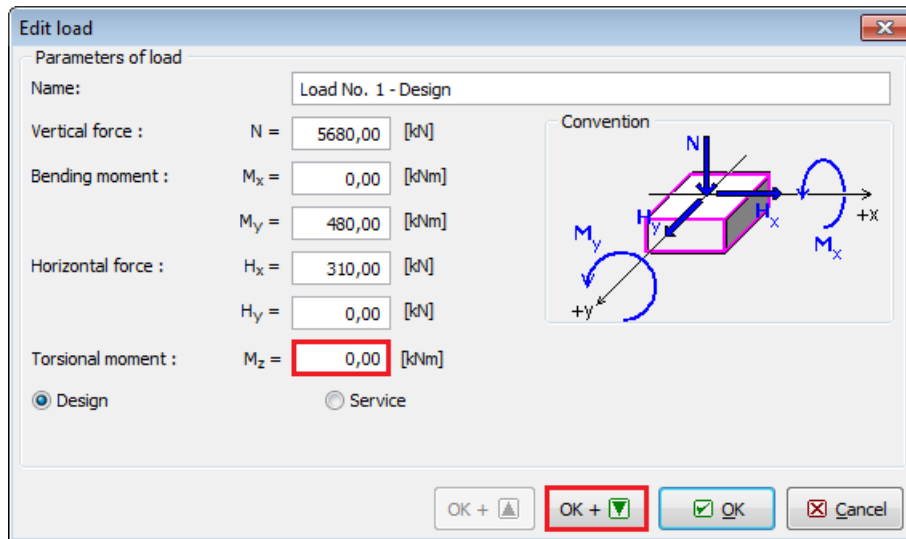
**Longitudinal reinforcement**

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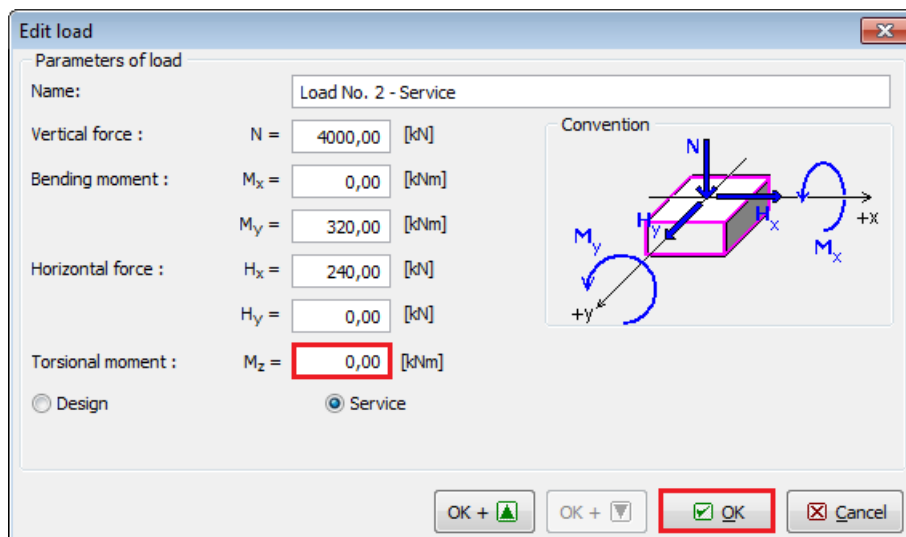
**B500**  
 $f_{yk} = 500,00$  MPa

“Material” frame

Subsequently, we will define the load. Design load is applied to the dimensioning of individual piles in the group and the determination of the internal forces curves, whilst service load is used for the calculation of deformations.

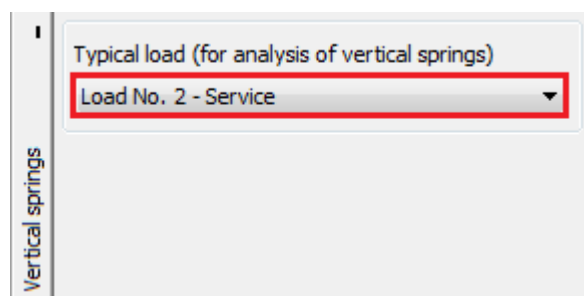


*Dialogue window "Edit load" – Design load*



*Dialogue window "Edit load" – Service load*

In the "Vertical springs" frame we will select the so-called typical load, which is used to calculate the stiffness of vertical springs. In our case we will choose the "Load No. 2 – Service" option.



*"Vertical springs" – Typical load*

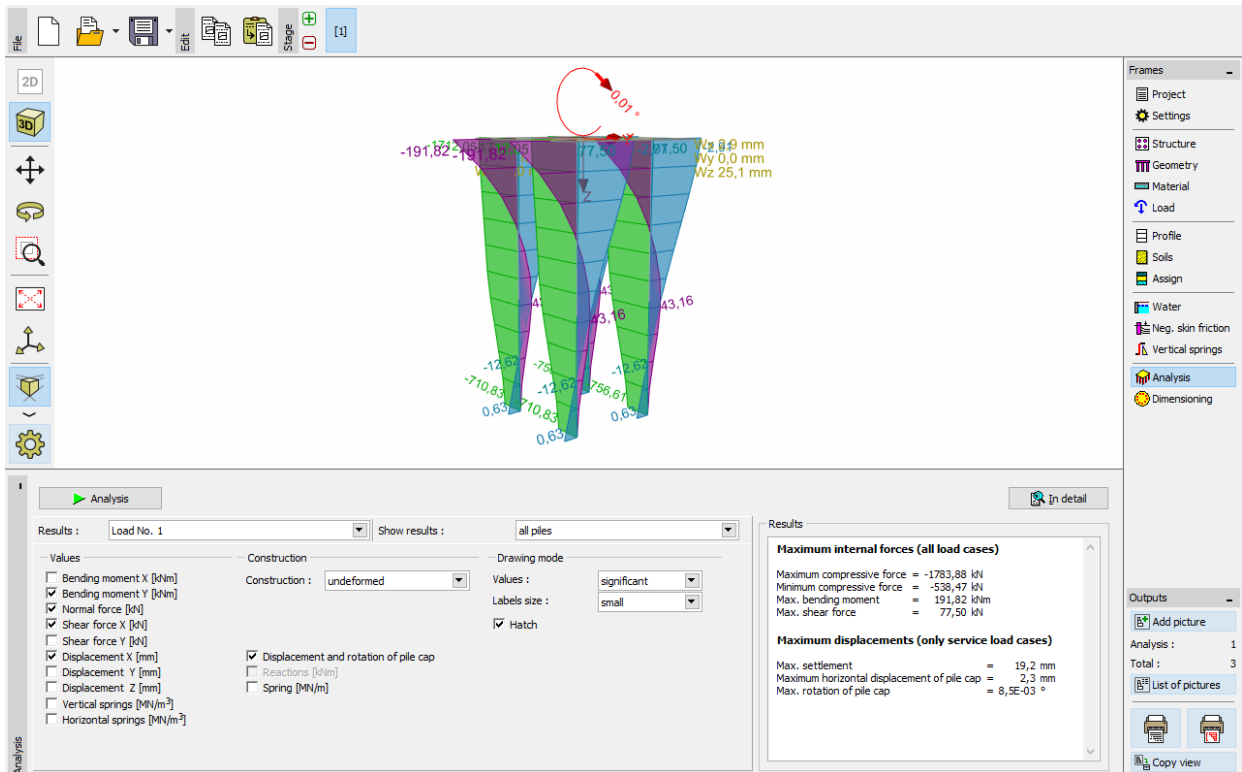
*Note: In the case of the Typical load option, the service (characteristic) load should be applied, which best characterises the structure behaviour (for more details visit Help – F1). The procedure for the computation of the vertical springs is as follows:*

- a) The calculated load is distributed among individual piles.*
- b) The size of vertical springs on the pile skin and at the base is determined for individual piles, depending on the load and soil parameters.*

*The effect of the load on the calculated stiffness is significant – for example, the stiffness of the spring at the base is always zero for a tensioned pile. For that reason, it may be advantageous in some cases to carry out the calculation several times for various typical loads.*

## Analysis: Spring Method

In the “Analysis” frame we will carry out the assessment of the pile group for the initial settings (the **constant** modulus of subsoil horizontal reaction) and will display the results with internal force curves.



“Analysis” frame – Spring Method (constant modulus of subsoil reaction)

*Note: The stiffness of piles in the group is automatically modified according to their locations. Piles on the edge and inside the group have the sizes of the horizontal stiffness and shear stiffness of springs reduced in comparison with a single pile. Springs on pile bases are not reduced (for more details visit Help – F1).*

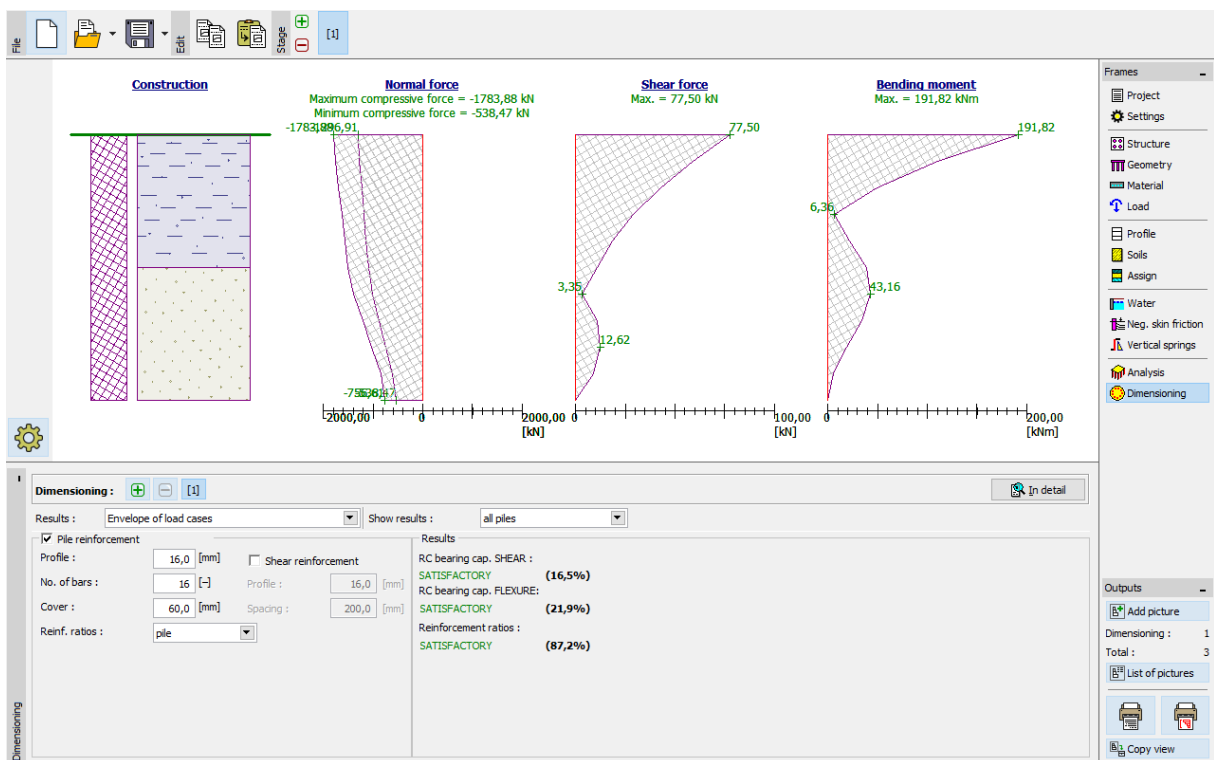
The results of the analysis for the initial settings (for maximum deformation) are as follows:

- Maximum settlement:  $19.2 \text{ mm}$ ;
- Max. horizontal displacement of pile cap:  $2.3 \text{ mm}$ ;
- Maximum rotation of pile cap:  $8.5 \cdot 10^{-3} \text{ }^\circ$ .

## Dimensioning

Subsequently we will move on to the “Dimensioning” frame and, similarly to the chapter 13. *Analysis of horizontal bearing capacity of single pile*, we will propose and assess the main structural reinforcement of the piles. We will consider identical reinforcement ratio for all piles in the group – **16 pcs Ø 16 mm** and the minimum concrete cover of **60 mm**, according to the exposure grade XC1.

The reinforcement ratio for a generally loaded pile group is in this case considered to be in accordance with CSN EN 1536:1999 (identically with that in *chapter 17*). In the program this option is set as a “pile” (for more details visit Help – F1).



“Dimensioning” frame – results for all piles in the group from the envelope of loading cases

We observe the utilisation of a cross-section of all piles in the group in terms of bending and the condition for the minimum reinforcement ratio for the overall envelope of load cases:

- RC pile bearing capacity (flexure): 21,9 % SATISFACTORY
- RC pile bearing capacity (shear): 16,5 % SATISFACTORY
- Reinforcement ratio: 87,2 % SATISFACTORY

$$(\rho = 0.410 > \rho_{\min} = 0.357 \%)$$

## Analysis results

The procedure for other analyses in the program is analogical with the procedure applied to the previous problems. We will always change the method of the calculation of the modulus of subsoil reaction in the “Settings” frame and will carry out the assessment of the pile group in the “Analysis” and “Dimensioning” frames. We will record the results in summary tables.

Modulus of subsoil reaction $k_h$ [ $MN/m^3$ ]	Compressive force (maximum, minimum) [ $kN$ ]	Maximum bending moment [ $kNm$ ]	Maximum shear force [ $kN$ ]
CONSTANT	-1783.88	191.82	77.50
	-538.47		
LINEAR (Bowles)	-1800.17	224.41	77.50
	-533.10		
according to CSN 73 1004	-1794.75	213.56	77.50
	-534.91		
according to VESIC	-1805.52	235.11	77.50
	-531.35		

*Summary of results (internal forces) – Verification of a pile group (spring method)*

Modulus of subsoil reaction $k_h$ [ $MN/m^3$ ]	Maximum settlement [ $mm$ ]	Max. horizontal displacement [ $mm$ ]	Max. rotation of pile cap [ $^\circ$ ]	RC pile bearing capacity [%]
CONSTANT	19.2	2.3	$8,5 \cdot 10^{-3}$	21.9
LINEAR (Bowles)	19.5	3.1	$1,4 \cdot 10^{-2}$	23.2
according to CSN 73 1004	19.4	2.9	$1,2 \cdot 10^{-2}$	22.8
according to VESIC	19.6	4.3	$1,5 \cdot 10^{-2}$	23.7

*Summary of results – displacements and dimensioning of a pile group*



## Conclusion

The values of the maximum settlement of the pile group, settlement displacements and the base slab rotation are within allowable limits.

It follows from the analysis results that the observed values of internal forces along the length of individual piles and the maximum deformations at pile heads in the group are slightly different, but the influence of the method selected for the calculation of the modulus of subsoil reaction  $k_h$  is not so essential.

The pile reinforcement cage proposed is satisfactory. The main condition for the reinforcement ratio of piles is also met.