# **Abutment verification**

# **Input data**

**Project** 

Date: 29.10.2015

**Settings** 

(input for current task)

Materials and standards

Abutment: EN 1992-1-1 (EC2)

Coefficients EN 1992-1-1: standard

Wall analysis

Active earth pressure calculation : Coulomb

Passive earth pressure calculation : Caquot-Kerisel
Earthquake analysis : Mononobe-Okabe
Shape of earth wedge : Calculate as skew

Allowable eccentricity: 0.333

Verification methodology: Safety factors (ASD)

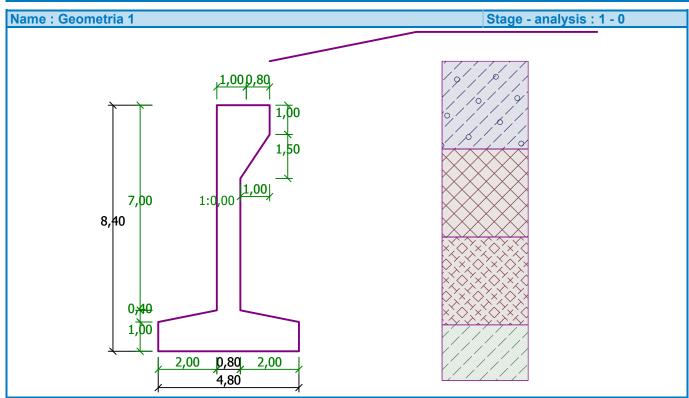
Safety factors					
Permanent design situation					
Safety factor for overturning :	SF <sub>o</sub> =	1,50	[-]		
Safety factor for sliding resistance :	SF <sub>s</sub> =	1,50	[-]		
Safety factor for bearing capacity :	SF <sub>b</sub> =	1,00	[-]		

## **Geometry of structure**

No.	Coordinate	Depth
NO.	X [m]	Z [m]
1	0,00	1,50
2	0,00	2,50
3	-1,00	4,00
4	-1,00	8,50
5	1,00	8,90
6	1,00	9,90
7	-3,80	9,90
8	-3,80	8,90
9	-1,80	8,50
10	-1,80	1,50
11	-0,80	1,50

The origin [0,0] is located at the most upper right point of the wall. Wall section area = 13,27 m<sup>2</sup>.

Length of bridge abutment = 5,00 m Length of abutment foundation = 5,40 m



# **Abutment wingwalls - hinged symmetric**

Wingwall thickness = 0,40 m
Length of wingwall behind closure wall = 4,00 m
Wingwall height = 4,00 m
Dist. of wingwall cut from c.w. = 2,00 m
Depth of wingwall cut = 4,00 m

#### **Material of structure**

Unit weight  $\gamma$  = 23,00 kN/m<sup>3</sup>

Analysis of concrete structures carried out according to the standard EN 1992-1-1 (EC2).

Concrete: C 20/25

Cylinder compressive strength  $f_{ck} = 20,00 \text{ MPa}$ Tensile strength  $f_{ctm} = 2,20 \text{ MPa}$ 

Longitudinal steel: B500

Yield strength  $f_{vk} = 500,00 \text{ MPa}$ 

## Soil parameters

Soil No. 1

Unit weight:  $\gamma = 19,00 \text{ kN/m}^3$ 

Stress-state: effective

Angle of internal friction :  $\phi_{ef} = 29,00^{\circ}$ Cohesion of soil :  $\phi_{ef} = 8,00^{\circ}$  kPa Angle of friction struc.-soil :  $\phi_{ef} = 8,00^{\circ}$  kPa Soil :  $\phi_{ef} = 15,00^{\circ}$  cohesionless

Saturated unit weight :  $\gamma_{sat} = 19,00 \text{ kN/m}^3$ 

Soil No. 2

Unit weight:  $\gamma = 19,00 \text{ kN/m}^3$ 

 $\begin{array}{lll} \text{Stress-state:} & \text{effective} \\ \text{Angle of internal friction:} & \phi_{\text{ef}} = 29,00\,^{\circ} \\ \text{Cohesion of soil:} & c_{\text{ef}} = 8,00\,\text{kPa} \end{array}$ 

Angle of friction struc.-soil :  $\delta$  = 15,00 ° Soil : cohesionless

Saturated unit weight :  $\gamma_{sat} = 19,00 \text{ kN/m}^3$ 

Soil No. 3

Unit weight:  $\gamma = 19.00 \text{ kN/m}^3$ 

Stress-state : effective

Angle of internal friction :  $\phi_{ef} = 29,00^{\circ}$  Cohesion of soil :  $c_{ef} = 8,00^{\circ}$  kPa Angle of friction struc.-soil :  $\delta = 15,00^{\circ}$  cohesionless

Saturated unit weight :  $\gamma_{sat} = 19,00 \text{ kN/m}^3$ 

Soil No. 4

Unit weight:  $\gamma = 19,00 \text{ kN/m}^3$ 

 $\begin{array}{lll} \text{Stress-state:} & \text{effective} \\ \text{Angle of internal friction:} & \phi_{\text{ef}} = 29,00 \ ^{\circ} \\ \text{Cohesion of soil:} & c_{\text{ef}} = 8,00 \ \text{kPa} \\ \text{Angle of friction struc.-soil:} & \delta = 15,00 \ ^{\circ} \\ \text{Soil:} & \text{cohesionless} \end{array}$ 

Saturated unit weight :  $\gamma_{sat} = 19,00 \text{ kN/m}^3$ 

#### Load case, bridge load

Type of load case: construction state.

## Geological profile and assigned soils

No.	Layer [m]	Assigned soil	Pattern
1	3,00	Soil No. 1	
2	3,00	Soil No. 2	
3	3,00	Soil No. 3	
4	-	Soil No. 4	

#### **Foundation**

Type of foundation: soil from geological profile

# **Terrain profile**

Terrain behind construction has the slope 1: 5,00 (slope angle is 11,31 °). Embankment height is 1,00 m, embankment length is 5,00 m.

#### Water influence

Ground water table is located below the structure.

### Resistance on front face of the structure

Resistance on front face of the structure is not considered.

#### Settings of the stage of construction

Design situation : permanent

The wall is free to move. Active earth pressure is therefore assumed.

# **Verification No. 1 (Stage of construction 1)**

# Forces acting on construction

Name	F <sub>hor</sub>	App.Pt.	F <sub>vert</sub>	App.Pt.	Design
	[kN/m]	z [m]	[kN/m]	x [m]	coefficient
Weight - wall	0,00	-3,33	305,21	2,51	1,000
Weight - earth wedge	0,00	-2,51	59,38	3,47	1,000
Active pressure	172,56	-2,43	195,45	3,96	1,000

#### **Abutment check**

Verification for slip has not been performed.

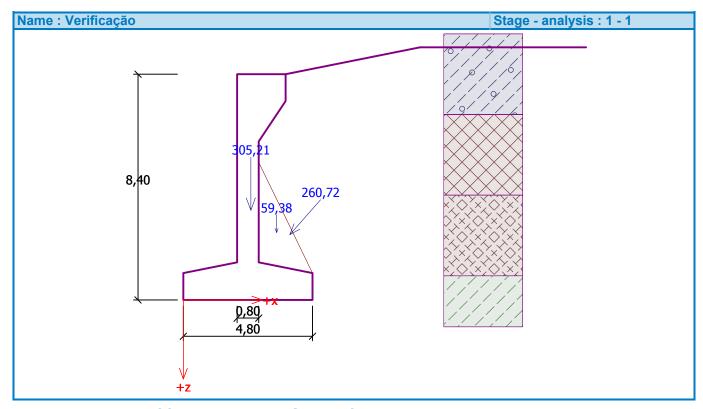
# Check for overturning stability

Resisting moment  $\dot{M}_{res}$  = 1615,54 kNm/m Overturning moment  $\dot{M}_{ovr}$  = 388,16 kNm/m

Safety factor = 4,16 > 1,50

Wall for overturning is SATISFACTORY

#### **Overall check - ABUTMENT is SATISFACTORY**



# **Bearing capacity of foundation soil (Stage of construction 1)**

Design load acting at the center of footing bottom

Design	load acting at the cel	iter or rooting botton	•			
No	Moment Norm. force		Shear Force	Eccentricity	Stress	
No.	[kNm/m]	[kN/m]	[kN/m]	[-]	[kPa]	
1	17,14	518,55	159,77	0,007	109,54	

Service load acting at the center of footing bottom

No	Moment	Norm. force	Shear Force	
No.	[kNm/m]	[kN/m]	[kN/m]	
1	17,14	518,55	159,77	

#### Verification of foundation soil

## **Eccentricity verification**

Max. eccentricity of normal force e = 0,007 Maximum allowable eccentricity e<sub>alw</sub> = 0,333

## **Eccentricity of the normal force is SATISFACTORY**

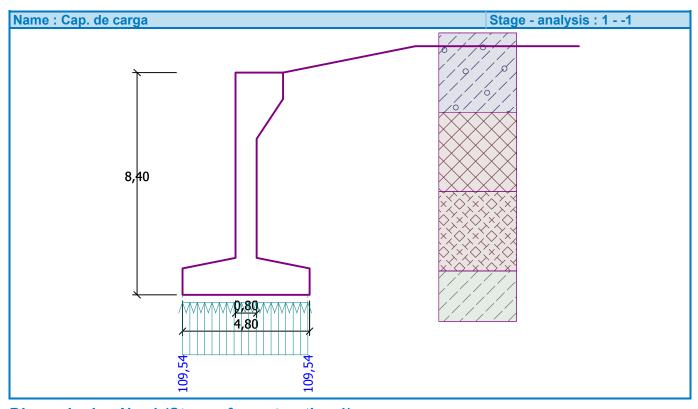
## Verification of bearing capacity

Max. stress at footing bottom  $\sigma$  = 109,54 kPa Bearing capacity of foundation soil R<sub>d</sub> = 240,00 kPa

Safety factor = 2,19 > 1,00

Bearing capacity of foundation soil is SATISFACTORY

# Overall verification - bearing capacity of found. soil is SATISFACTORY



# **Dimensioning No. 1 (Stage of construction 1)**

## Forces acting on construction

Name	F <sub>hor</sub>	App.Pt.	F <sub>vert</sub>	App.Pt.	Design
	[kN/m]	z [m]	[kN/m]	x [m]	coefficient
Weight - wall	0,00	-4,11	169,05	0,60	1,000
Active pressure	87,83	-1,76	23,54	0,80	1,000

## Dimensioning of abutment stem - input data:

Construction joint is designed from steel-reinforced concrete; design width 1m.

Bar diameter = 25,0 mm Number of bars = 12

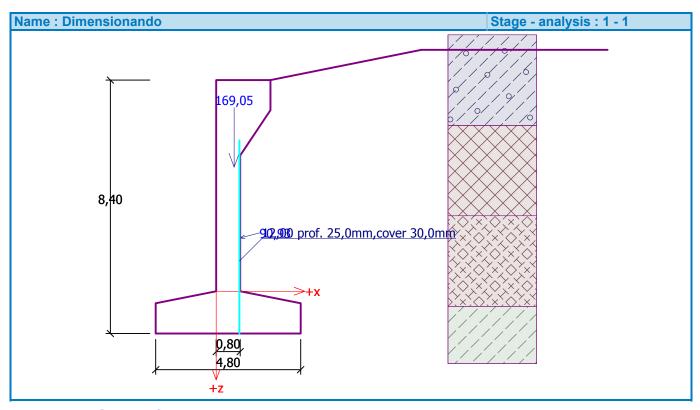
Reinforcement cover = 30,0 mm

Internal forces : M = 112,19 kNm/m; N = -192,58 kN/m; V = 87,83 kN/m Cross-section depth h = 0,80 m

## **Dimensioning of abutment stem - results:**

Reinforcement ratio  $\rho$  = 0,74 % > 0,13 % =  $\rho_{min}$  Position of neutral axis x = 0,50 m Ultimate shear force  $V_{Rd}$  = 366,18 kN/m > 87,83 kN/m =  $V_{Ed}$  Ultimate compressive force  $V_{Rd}$  = 3152,34 kN/m > 192,58 kN/m =  $V_{Ed}$  Ultimate moment  $V_{Rd}$  = 1836,48 kNm/m > 112,19 kNm/m =  $V_{Ed}$ 

## **Cross-section is SATISFACTORY.**



# Input data (Stage of construction 2)

 $a_2 = 0.20 \text{ m}$ 

## Load case, bridge load

Location

Type of load case : service state. **Forces generated by bridge**Vertical force  $F_s = 2000,00 \text{ kN}$ Horizontal force  $F_v = 0,00 \text{ kN}$ Location  $a_1 = 0,30 \text{ m}$ Depth v = 0,10 m **Forces due to transition slab**Vertical force  $F_s = 120,00 \text{ kN}$ Horizontal force  $F_v = -50,00 \text{ kN}$ 

Projeto: Shopping centro Black Rose

## Geological profile and assigned soils

No.	Layer [m]	Assigned soil	Pattern
1	3,00	Soil No. 1	
2	3,00	Soil No. 2	
3	3,00	Soil No. 3	
4	-	Soil No. 4	

#### **Foundation**

Type of foundation : soil from geological profile

## **Terrain profile**

Terrain behind construction has the slope 1: 5,00 (slope angle is 11,31 °). Embankment height is 1,00 m, embankment length is 5,00 m.

#### Water influence

Ground water table is located below the structure.

#### Resistance on front face of the structure

Resistance on front face of the structure is not considered.

## Settings of the stage of construction

Design situation : permanent

The wall is free to move. Active earth pressure is therefore assumed.

# **Verification No. 1 (Stage of construction 2)**

# Forces acting on construction

Name	F <sub>hor</sub>	App.Pt.	F <sub>vert</sub>	App.Pt.	Design
	[kN/m]	z [m]	[kN/m]	x [m]	coefficient
Weight - wall	0,00	-3,81	332,81	2,58	1,000
Weight - earth wedge	0,00	-2,51	59,38	3,47	1,000
Active pressure	224,88	-2,66	249,09	3,92	1,000
Abutment wingwalls	0,00	-8,00	54,28	5,50	1,000
Bridge reactions	0,00	-8,50	400,00	2,30	1,000
Appr. plate react.	10,00	-9,90	24,00	3,60	1,000

# **Abutment check**

Verification for slip has not been performed.

## Check for overturning stability

Resisting moment  $M_{res} = 3099,22 \text{ kNm/m}$ Overturning moment  $M_{ovr} = 644,78 \text{ kNm/m}$ 

Safety factor = 4.81 > 1.50

Wall for overturning is SATISFACTORY

**Overall check - ABUTMENT is SATISFACTORY** 

# **Bearing capacity of foundation soil (Stage of construction 2)**

Design load acting at the center of footing bottom

No.	Moment Norm. force [kNm/m] [kN/m]		Shear Force [kN/m]	Eccentricity [–1	Stress [kPa]
1	33,47	1036,62	217,48	0,007	218,91

Service load acting at the center of footing bottom

No.	Moment	Norm. force	Shear Force	
NO.	[kNm/m]	[kN/m]	[kN/m]	
1	33,47	1036,62	217,48	

#### **Verification of foundation soil**

## **Eccentricity verification**

Max. eccentricity of normal force e = 0,007Maximum allowable eccentricity  $e_{alw} = 0,333$ 

#### **Eccentricity of the normal force is SATISFACTORY**

#### Verification of bearing capacity

Max. stress at footing bottom  $\sigma$  = 218,91 kPa Bearing capacity of foundation soil R<sub>d</sub> = 240,00 kPa

Safety factor = 1,10 > 1,00

Bearing capacity of foundation soil is SATISFACTORY

#### Overall verification - bearing capacity of found. soil is SATISFACTORY

# **Dimensioning No. 1 (Stage of construction 2)**

# Forces acting on construction

Name	F <sub>hor</sub>	App.Pt.	F <sub>vert</sub>	App.Pt.	Design
	[kN/m]	z [m]	[kN/m]	x [m]	coefficient
Weight - wall	0,00	-4,62	196,65	0,71	1,000
Active pressure	128,68	-2,07	34,48	0,83	1,000
Abutment wingwalls	0,00	-6,60	54,28	3,50	1,000
Bridge reactions	0,00	-7,10	400,00	0,30	1,000
Appr. plate react.	10,00	-8,50	24,00	1,60	1,000

#### **Dimensioning of abutment stem - input data:**

Construction joint is designed from steel-reinforced concrete; design width 1m.

Bar diameter = 25,0 mm Number of bars = 12 Reinforcement cover = 30,0 mm

Internal forces : M = 118,41 kNm/m; N = -709,41 kN/m; V = 138,68 kN/m

Cross-section depth h = 0.80 m

### **Dimensioning of abutment stem - results:**

Reinforcement ratio  $\rho = 0.74 \% > 0.13 \% = \rho_{min}$ 

Position of neutral axis x = 0.67 m

Ultimate shear force  $V_{Rd} = 443,70 \text{ kN/m} > 138,68 \text{ kN/m} = V_{Ed}$  Ultimate compressive force  $N_{Rd} = 6692,26 \text{ kN/m} > 709,41 \text{ kN/m} = N_{Ed}$  Ultimate moment  $M_{Rd} = 1116,99 \text{ kNm/m} > 118,41 \text{ kNm/m} = M_{Ed}$ 

**Cross-section is SATISFACTORY.**