

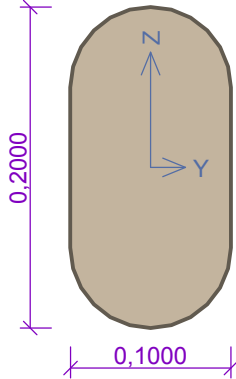
## Demo01

### Calculation factors

According to standard CSN EN 1992-1-1.

### Geometry

Column cross-section:



Cross-section dimension	
cross-section height	$h = 0,2000 \text{ m}$
cross-section width	$b = 0,1000 \text{ m}$

Slab thickness  $h_s = 0,250 \text{ m}$

Column type - internal

### Materials

Concrete: C 20/25, Longitudinal reinforcement: B500, Stirrups: B500

Concrete C 20/25

Strength in compression of concrete  $f_{c,k} = 20,0 \text{ MPa}$

Strength in tension of concrete  $f_{c,t} = 2,2 \text{ MPa}$

Concrete Young modulus  $E_{c,m} = 29000,0 \text{ MPa}$

Longitudinal reinforcement B500

Steel strength  $f_{t,k} = 500,0 \text{ MPa}$

Steel Young modulus  $E = 200000,0 \text{ MPa}$

Longitudinal reinforcement B500

Steel strength  $f_{t,k} = 500,0 \text{ MPa}$

Steel Young modulus  $E = 200000,0 \text{ MPa}$

### Load

Shear force  $V_{Ed} = 300,00 \text{ kN}$

Bending moment in direction x  $M_{Ed,x} = 0,00 \text{ kNm}$

Bending moment in direction y  $M_{Ed,y} = 0,00 \text{ kNm}$

Axial force in slab  $N_{Ed,x} = 0,00 \text{ kN}$  acting on width 1,000m

Axial force in slab  $N_{Ed,y} = 0,00 \text{ kN}$  acting on width 1,000m

### Reinforcement

Slab reinforcement in direction of axis x:  $10,0 \times \varnothing 12,0 \text{ mm/m}$ , cover 30,0 mm

Slab reinforcement in direction of axis y:  $10,0 \times \varnothing 12,0 \text{ mm/m}$ , cover 42,0 mm

### Shear area

Shear reinforcement not set

### Critical circumference table

column distance [m]	circumference [m]	$v_{Ed}$ [MPa]	$v_{Rd}$ [MPa]	Result
0	0,514	2,805	3,68	Pass
0,416	3,128	0,461	0,527	Pass

## Detailed check

Effective slab thickness:

$$d_x = h - c_x - 0,5 \times \phi_s = 0,25 - 0,03 - 0,5 \times 0,012 = 0,214 \text{ m}$$

$$d_y = h - c_y - 0,5 \times \phi_s = 0,25 - 0,042 - 0,5 \times 0,012 = 0,202 \text{ m}$$

$$d = 0,5 \times (d_x + d_y) = 0,5 \times (0,214 + 0,202) = 0,208 \text{ m}$$

Factor  $\beta$ :

$$\beta = 1$$

Maximum resistance on column circumference  $v_{Rd,max}$ :

$$v = 0,6 \times (1 - f_{ck} / 250) = 0,6 \times (1 - 20 / 250) = 0,552$$

$$v_{Rd,max} = 0,5 \times v \times f_{cd} = 0,5 \times 0,552 \times 13,33 = 3,68 \text{ MPa}$$

Shear stress on column circumference  $v_{Ed,max}$ :

$$v_{Ed,max} = \beta \times V_{Ed} / (u_0 \times d) = 1 \times 300 / (0,514 \times 0,208) = 2,805 \text{ MPa}$$

**$v_{Ed,max} \leq v_{Rd,max} \Rightarrow$  Pass**

Concrete resistance  $v_{Rd,c}$ :

$$C_{Rd,c} = 0,18 / \gamma_C = 0,18 / 1,5 = 0,12$$

$$k = \min(1 + \sqrt{(200 / d)}; 2) = \min(1 + \sqrt{(200 / 0,208)}; 2) = 1,981$$

$$A_{sx} = 2,5 \times \pi \times \phi_s^2 = 2,5 \times 3,142 \times 0,012^2 = 0,00113 \text{ m}^2$$

$$\rho_{lx} = A_{sx} / (1 \times d) = 0,00113 / (1 \times 0,208) = 0,00544$$

$$A_{sy} = 2,5 \times \pi \times \phi_s^2 = 2,5 \times 3,142 \times 0,012^2 = 0,00113 \text{ m}^2$$

$$\rho_{ly} = A_{sy} / (1 \times d) = 0,00113 / (1 \times 0,208) = 0,00544$$

$$\rho_l = \sqrt{(\rho_{lx} \times \rho_{ly})} = \sqrt{(0,00544 \times 0,00544)} = 0,00544$$

$$v_{min} = 0,035 \times k^{1,5} \times \sqrt{f_{ck}} = 0,035 \times 1,981^{1,5} \times \sqrt{20} = 0,436 \text{ MPa}$$

$$v_{Rd,c} = \max(C_{Rd,c} \times k \times \sqrt[3]{(100 \times \rho_l \times f_{ck})}; v_{min}) = \max(0,12 \times 1,981 \times \sqrt[3]{(100 \times 0,00544 \times 20)}; 0,436) = 0,527 \text{ MPa}$$

Length of check circumference, which meets the condition  $v_{Rd,c} \geq v_{Ed}$  :

$$u_{out} = \beta \times V_{Ed} / (v_{Rd,c} \times d) = 1 \times 300 / (0,527 \times 0,208) = 2,739 \text{ m}$$

this circumference lies at distance 0,354 m from the column edge

### Check of circumference no. 1 at distance 0,416 m from the column edge

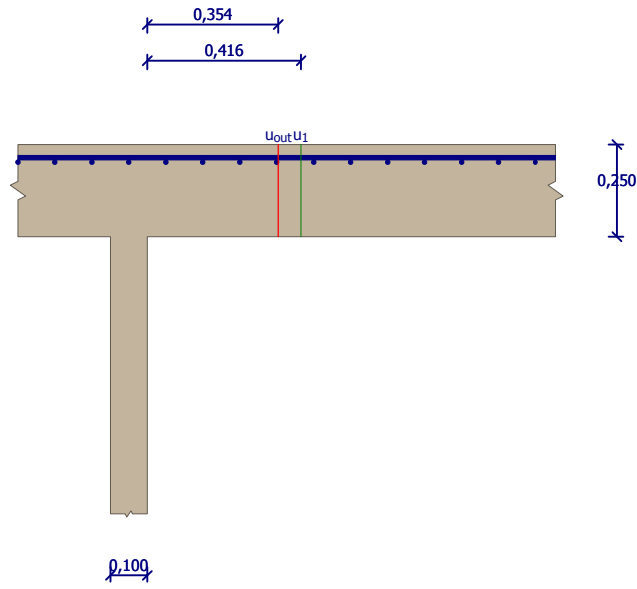
Shear stress from load

$$v_{Ed} = \beta \times V_{Ed} / (u_0 \times d) = 1 \times 300 / (3,128 \times 0,208) = 0,461 \text{ MPa}$$

### Slab resistance in punching passes



Front view



Top view

